

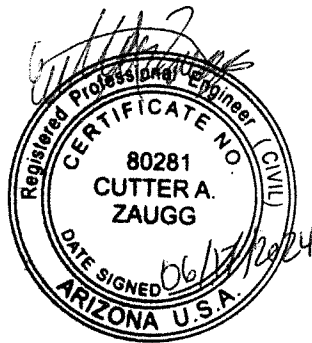


## CONSULTING ENGINEERS, INC.

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APN# 304-91-045J & 304-91-045K  
(A.C.E. Job # 97839)

June 17, 2024



Prepared by: Allen Consulting Engineers, Inc.  
Cutter A. Zaugg, P.E.

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4111 E. Valley Auto Drive, #103, Mesa, Arizona 85207  
Phone: 480-844-1666  
Email: [ace@allenconsultengr.com](mailto:ace@allenconsultengr.com)

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## **INTRODUCTION**

This drainage report for 26026 S. Ellsworth Road will outline the hydrology and hydraulic calculations and procedures for the further development of this site. This report will be submitted per the requirements outlined in the current version of the Town of Queen Creek Subdivision Ordinance as well as the current version of the Maricopa County Drainage Policies and Standards, Drainage Design Manuals for Maricopa County, Volume 1 – Hydrology and Volume 2 – Hydraulics, and Section 1205 of the Maricopa County Zoning Ordinance per form 712A Drainage Report #10. The results of this study will be used to ensure the site structures are free from flooding for the 100 year storm event. All designs in this report will be developed in accordance with the current Maricopa County regulations, standards and policies.

## **LOCATION**

The development is located within a portion in the southeast quarter of Section 33, Township 2 South, Ranch 7 East at the intersection of Ellsworth Road and San Tan Boulevard in Queen Creek on 4.19 acres. Refer to Figure 1 for a location map.

## **SITE DESCRIPTION**

The site consists of vacant lots with existing retention basins along the eastern property line for offsite retention purposes. The site slopes from south to north at approximately 0.008’/’.

## **PROPOSED DEVELOPMENT**

Four apartment buildings are proposed for the site with onsite retention provided. The retention basin system for the project will be divided between onsite and offsite flows. The onsite retention basins can be found along the western and southern property lines with onsite grading and valley gutters directing the flow to these areas. Offsite flows from Ellsworth Road enter the site via existing scuppers which some will remain and others will be relocated or proposed.

## **FEMA FLOODPLAIN CLASSIFICATION**

This site lies in flood zone “X” per FIRM Panel 04013C3135L dated 10-16-2013. Therefore, there is not any portion of this site that will lie within any designated floodplain and no flood insurance will be required. Please refer to Figure 3 for a FIRM map.

## **OFFSITE DRAINAGE DESCRIPTION**

The site accepts and retains offsite flow from Ellsworth Road via existing scuppers. The required retention volume of the half street was calculated and the new development will continue providing onsite retention for the offsite flows.

## **ONSITE DRAINAGE CONVEYANCE**

Drainage runoff generated from the proposed development will be directed to the western and southern retention basins via valley gutters and onsite grading.

Weighted runoff “C” coefficients were calculated and used to determine the peak flows. Refer to Appendix A for weighted “C” coefficient calculations. Finished floors shall be set at the higher elevation of either 14-inches above the low lot outfall, or 12-inches above the 100-year water surface elevation.

## **RETENTION BASIN DESIGN**

The proposed retention basin systems for this development will be designed to accommodate the volume of runoff generated by the 100-year 2-hour duration storm event. Any retention basins adjacent to local streets will be designed with a maximum ponding depth of 1.0 foot with 6:1 side slopes. The retention volume required has been divided between onsite and offsite drainage areas and weighted coefficients. On lot retention basins may be designed with a maximum of 4:1 side slopes. The basins along the western and southern property lines are 2.5 feet deep with Maxwell Plus drywells installed to drain the basins within 36 hours. Refer to the G&D plan for the location of the Maxwell Plus drywells. Storm water retention for the development adjacent to Ellsworth Road will be retained via above ground retention basins 1 foot deep. Refer to Appendix A for retention calculations.

## **RETENTION BASIN DRAIN TIME**

All retention basins will be required to drain within 36 hours per Section 3.11.6 of the Maricopa County Drainage Policies and Standards Manual. Drywells are proposed in retention basins deeper than 1-ft. Basins 2 and 3 are connected via a bleed off pipe and therefore only require 1 drywells between the 2 basins. An assumed rate of 0.1 CFS is used to determine the # of drywells required. Basins 1-ft deep and shallower will drain via natural percolation with no drywells required. Refer to Appendix B for drain time calculations.

## **SUMMARY AND CONCLUSIONS**

- 1.) Finished floor elevations will be set at the higher elevation of either 12” above the 100-year water surface elevation and 14” above the low lot outfall.
- 2.) Offsite flows are retained onsite along Ellsworth Road.
- 3.) Storm water runoff from the site will be retained within onsite above ground retention basins. These retention basins will provide retention for the 100-year 2-hour storm event in accordance with Flood Control District of Maricopa County Drainage Design Manual.
- 4.) This grading and drainage plan and drainage report are based on the standards and policies of the Maricopa County Flood Control Department.

**REFERENCES:**

Flood Control District of Maricopa County, Drainage Design Manual for Maricopa County – Volume II, Hydraulics, August 2013.

Flood Control District of Maricopa County, Drainage Design Manual for Maricopa County – Volume I, Hydrology, August 2013.

Flood Control District of Maricopa County, Drainage Policies and Standards Manual, January 2007.

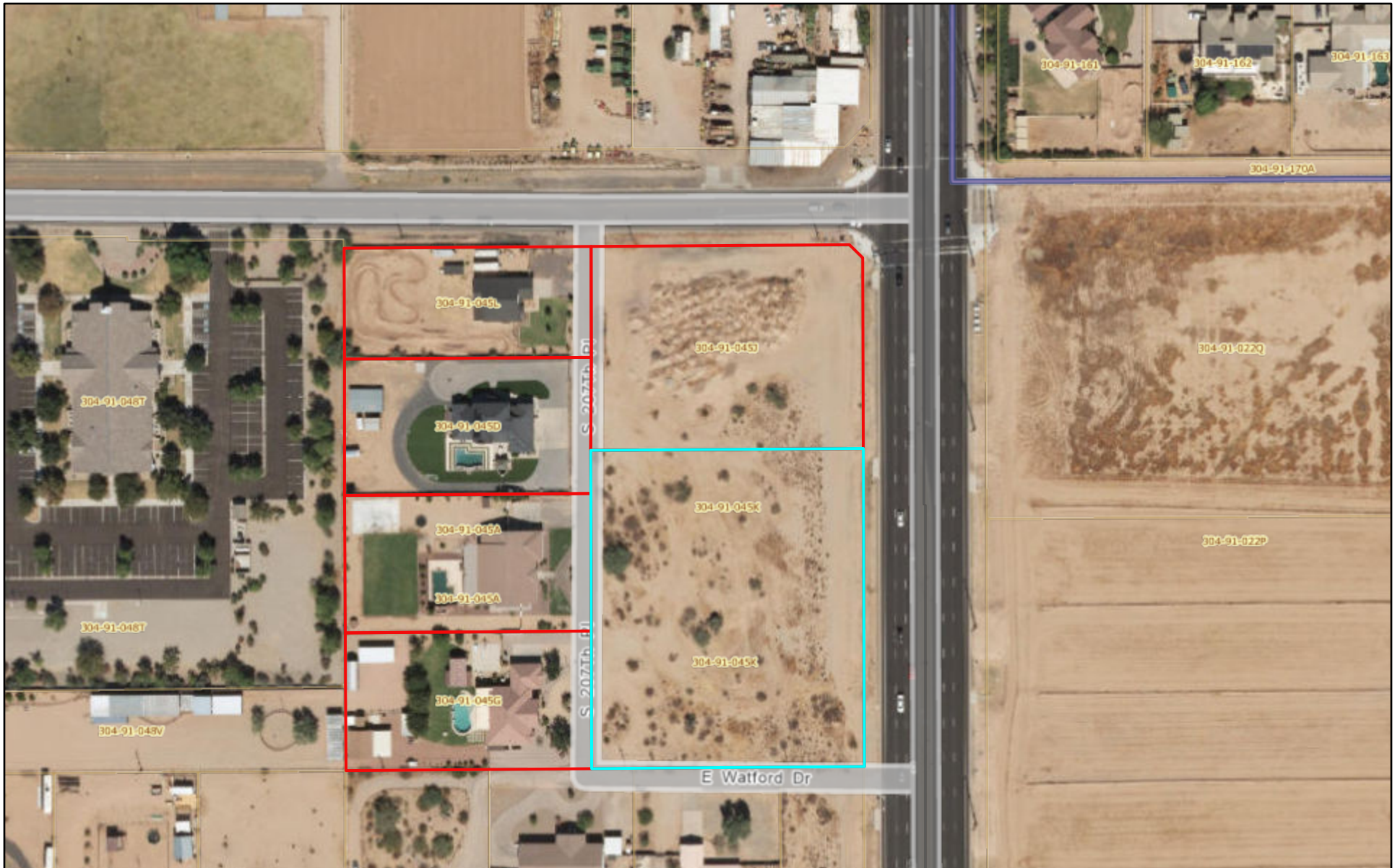
Town of Queen Creek, Subdivision Ordinance, October 2008.

# FIGURES

- Figure 1: Location Map
- Figure 2: Aerial Map
- Figure 3: FEMA FIRM Map
- Figure 4: Proposed Drainage Map

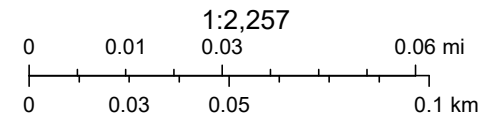


# FIGURE 2 - SITE AERIAL MAP



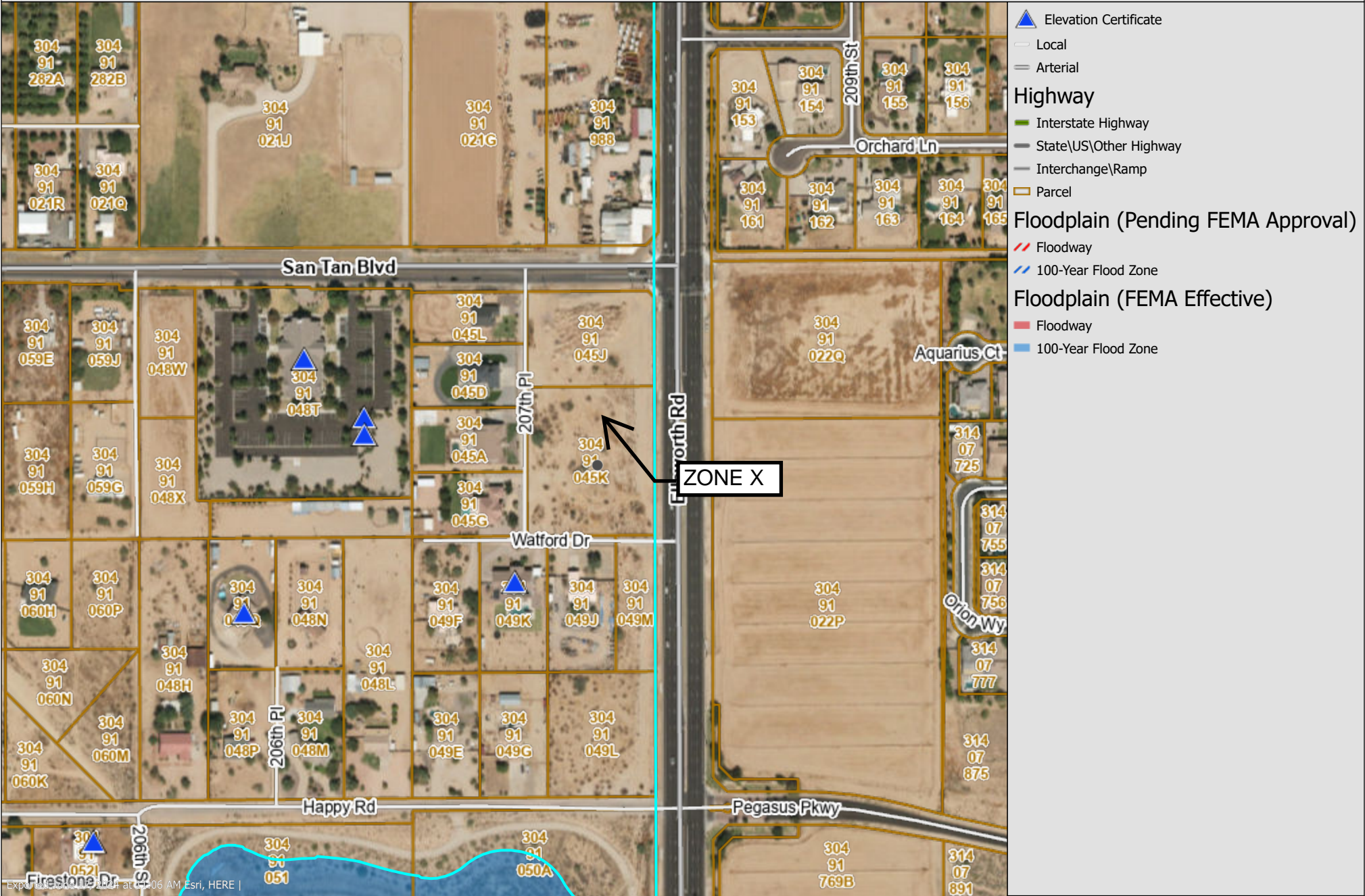
June 17, 2024

 Override 1



Maricopa County GIO, Maricopa County Assessor's Office

# FIGURE 3 - FEMA FLOODPLAIN

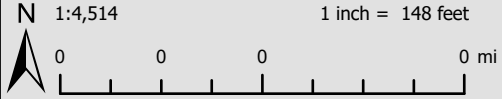


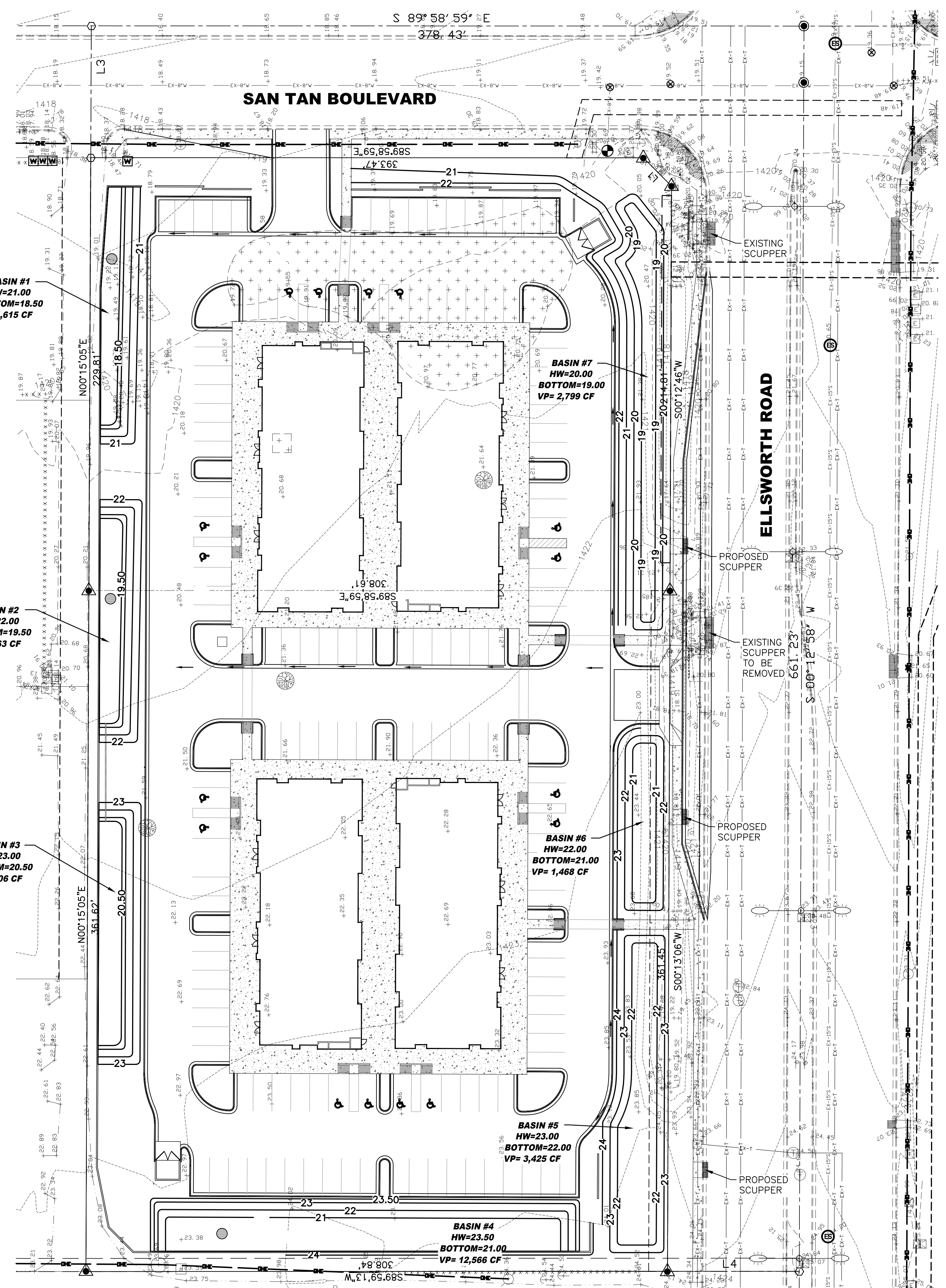
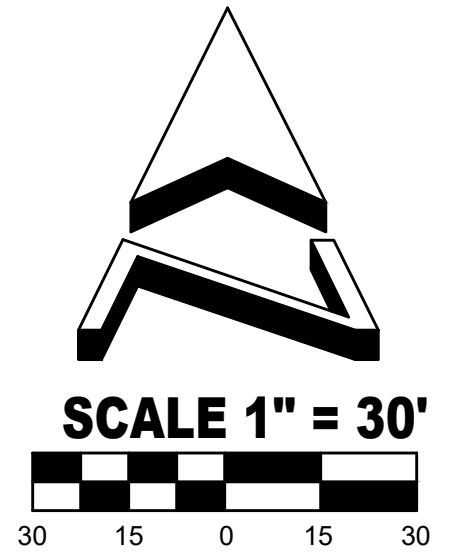
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**Flood Control District of Maricopa County**  
 2801 W Durango St  
 Phoenix, AZ 85009  
 (602) 506-2419  
<http://www.fcd.maricopa.gov>

**Unofficial Document**  
 This document cannot be used for floodplain determinations. Current studies, erosion setbacks and other factors may also affect the floodplain status of the property. The information shown for pending floodplains are the best technical information available at this time to determine the 1% chance flood and are subject to change.





# PRELIMINARY DRAINAGE MAP

**A PORTION OF THE SOUTHEAST QUARTER OF SECTION 33,  
TOWNSHIP 2 SOUTH, RANGE 7 EAST OF THE G.&S.R.B.&M.,  
MARICOPA COUNTY, ARIZONA.**

## RETENTION CALCULATIONS - ONSITE

VOLUME REQUIRED = C X A X (D/12)

A=182,409 SF

**WEIGHTED C COEFFICIENT**  
 CONCRETE/ASPHALT(0.95) - 131,973 SF  
 LANDSCAPING(0.50) - 50,436 SF

$(131,973 * 0.95) + (50,436 * 0.50) = 0.83$   
182,409 SF

C=0.83  
D=2.2"  
A=182,409 SF

VOLUME REQUIRED = LOT AREA X C VALUE X (2.2/12)  
 = 182,409 X 0.83 X (2.2/12)  
 = 27,757 C.F. NEEDED

BASIN 1 =  $\frac{(2,572 + 1,120)}{2} \times 2.5' = 4,615$  CF  
 BASIN 2 =  $\frac{(2,745 + 1,225)}{2} \times 2.5' = 4,963$  CF  
 BASIN 3 =  $\frac{(3,104 + 1,461)}{2} \times 2.5' = 5,706$  CF  
 BASIN 4 =  $\frac{(6,325 + 3,728)}{2} \times 2.5' = 12,566$  CF

VOLUME PROVIDED = 27,850 C.F. PROVIDED

## RETENTION CALCULATIONS - OFFSITE

VOLUME REQUIRED = C X A X (D/12)

A = 40,329 SF

**WEIGHTED C COEFFICIENT**  
 CONCRETE/ASPHALT(0.95) - 35,992 SF  
 LANDSCAPING(0.50) - 4,337 SF

$(35,992 * 0.95) + (4,337 * 0.50) = 0.90$   
40,329 SF

C=0.90  
D=2.2"  
A=40,329 SF (HALF STREET ALONG ELLSWORTH)

VOLUME REQUIRED = AREA X C VALUE X (2.2/12)  
 = 40,329 X 0.90 X (2.2/12)  
 = 6,654 C.F. NEEDED

BASIN 5 =  $\frac{(4,169 + 2,680)}{2} \times 1.0' = 3,425$  CF  
 BASIN 6 =  $\frac{(1,882 + 1,053)}{2} \times 1.0' = 1,468$  CF  
 BASIN 7 =  $\frac{(3,765 + 1,833)}{2} \times 1.0' = 2,799$  CF

VOLUME PROVIDED = 7,692 C.F. PROVIDED



<p><b>ALLEN CONSULTING ENGINEERS, INC.</b></p>		4111 E. VALLEY AUTO DRIVE, SUITE 103 MESA, ARIZONA 85206 PHONE (480) 844-1666 E-MAIL: ace@allenconsultengr.com	
		APN'S 304-91-045J & 304-91-045K 26026 S. ELLSWORTH ROAD QUEEN CREEK, AZ 85142 DRAINAGE MAP	
JOB NUMBER	97839	SHEET	1 OF 1
DRAWING	G&D	CHECKED BY	
DRAFTSMAN		DATE	06/17/2024

# APPENDIX A

## Retention Calculations

### ONSITE RETENTION REQUIRED

$$VR = C * A * (D/12)$$

$$A = \text{Lot Area} = 182,409 \text{ SF}$$

Weighted C Coefficient

$$\text{Concrete/Asphalt (0.95)} = 131,973 \text{ SF}$$

$$\text{Landscaping (0.50)} = 50,436 \text{ SF}$$

$$C = ((131,973 * 0.95) + (50,436 * 0.50)) / (182,409 \text{ SF}) = 0.83$$

$$VR = (0.83) * (182,409) * (2.2/12)$$

$$\text{Volume Required} = 27,757 \text{ CF}$$

### ONSITE RETENTION PROVIDED

$$\text{BASIN 1} = ((2,572 + 1,120)/2) * 2.5' \\ = 4,615 \text{ CF}$$

$$\text{BASIN 2} = ((2,745 + 1,225)/2) * 2.5' \\ = 4,963 \text{ CF}$$

$$\text{BASIN 3} = ((3,104 + 1,461)/2) * 2.5' \\ = 5,706 \text{ CF}$$

$$\text{BASIN 4} = ((6,325 + 3,728)/2) * 2.5' \\ = 12,566 \text{ CF}$$

$$\text{Volume Provided} = 27,850 \text{ CF}$$

### OFFSITE RETENTION REQUIRED

$$VR = C * A * (D/12)$$

$$A = \text{Half Street Area} = 40,329 \text{ SF}$$

Weighted C Coefficient

$$\text{Concrete/Asphalt (0.95)} = 35,992 \text{ SF}$$

$$\text{Landscaping (0.50)} = 4,337 \text{ SF}$$

$$C = ((35,992 * 0.95) + (4,337 * 0.50)) / (40,329 \text{ SF}) = 0.90$$

$$VR = (0.90) * (40,329) * (2.2/12)$$

$$\text{Volume Required} = 6,654 \text{ CF}$$

### OFFSITE RETENTION PROVIDED

$$\text{BASIN 5} = ((4,169 + 2,680)/2) * 1.0' \\ = 3,425 \text{ CF}$$

$$\text{BASIN 6} = ((1,882 + 1,053)/2) * 1.0' \\ = 1,468 \text{ CF}$$

$$\text{BASIN 7} = ((3,765 + 1,833)/2) * 1.0' \\ = 2,799 \text{ CF}$$

$$\text{Volume Provided} = 7,692 \text{ CF}$$

# APPENDIX B

## Basin Drain Time Calculations

Drywell Percolation Rate (Assumed) = 0.1 cfs

Basin 1 Volume = 4,615 CF

Basin 1 Drain Time = (4,615 CF) / (0.1 cfs) = 46,150 seconds = **12.82 Hours**

Basin 2+3 Volume = 10,669 CF

Basin 1 Drain Time = (10,669 CF) / (0.1 cfs) = 106,690 seconds = **29.64 Hours**

Basin 4 Volume = 12,566 CF

Basin 1 Drain Time = (12,566 CF) / (0.1 cfs) = 125,560 seconds = **34.88 Hours**

**Basins 5, 6 and 7 are 1 foot deep and do not require drywells.**